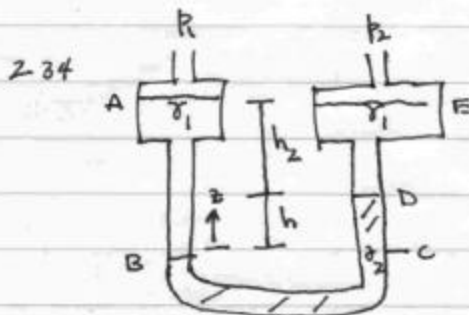




Since $A_2 \ll A_1$, we assume that the fluid heights open to the air are at the same level, i.e. $h_A = h_E = h + h_2$. Compute the pressure at the points A, B, C, D, E:

A: $p = p_1$

B: $p = p_1 + \gamma_1(h_2 + h)$ from the head of fluid 1

C: $p = p_1 + \gamma_1(h_2 + h)$ since the connecting fluid is homogeneous

D: $p = p_1 + \gamma_1(h_2 + h) - \gamma_2 h$ from the head of fluid 2

E: $p = p_1 + \gamma_1(h_2 + h) - \gamma_2 h - \gamma_1 h_2$ from the head of fluid 1

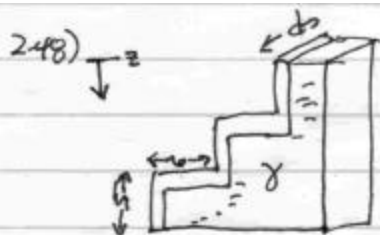
But point E is also at pressure p_2 :

$$p_2 = p_1 + \gamma_1(h_2 + h) - \gamma_2 h - \gamma_1 h_2$$

Rearranging:

$$h = \frac{p_1 - p_2}{\gamma_2 - \gamma_1}$$

Note: h does not "blow up" if the entire fluid is homogeneous ($\gamma_2 = \gamma_1$). Rather, the assumption that the top fluid levels are equal becomes invalid.



$$d = 3 \text{ ft}$$

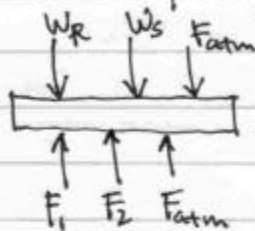
$$\gamma = 150 \text{ lb/ft}^3$$

$$W = 10 \text{ in} = 5/6 \text{ ft}$$

$$W_R = 85 \text{ lb}$$

$$h = 8 \text{ in} = 2/3 \text{ ft}$$

Draw a simple free body diagram:



where F_1 = force from cement under step 1

F_2 = force from cement under step 2

F_{atm} = force from the atmosphere

W_R = weight of the riser

W_S = weight of the sand

For the steps to be stationary:

$$W_R + W_S + F_{atm} = F_1 + F_2 + F_{atm}$$

$$W_S = F_1 + F_2 - W_R$$

Compute the force on each step from the hydrostatic eqn with zero at the open top step

$$F_1 = p_1 A = (2\gamma h)(Wd) \quad F_2 = (\gamma h)(Wd)$$

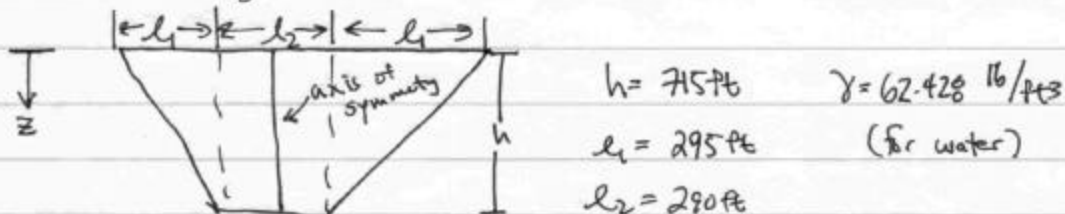
So

$$W_S = 3\gamma h Wd - W_R$$

$$W_S = 3\left(\frac{150 \text{ lb}}{\text{ft}^3}\right)\left(\frac{2}{3} \text{ ft}\right)\left(\frac{5}{6} \text{ ft}\right)(3 \text{ ft}) - 85 \text{ lb}$$

$$W_S = 665 \text{ lb}$$

2.72) Since we are only concerned with the horizontal force on the dam, we can just compute the pressure force on the projection in (b). The subscript R is the rectangle and T is the triangle.



Note: Atmospheric pressure cancels out on both sides of the dam.

Method 1: Use the centroids to compute the forces:

$$c_T = h/3, \quad c_R = h/2$$

The force on the triangle is the pressure @ the centroid times

the area of the triangle:

$$F_T = \left(\underset{\substack{\uparrow \\ \text{pressure at} \\ c_T}}{\frac{\gamma h}{3}} \right) \left(\underset{\sim \text{Area}}{\frac{1}{2} l_2 h} \right) = \frac{\gamma l_2 h^2}{6}$$

Likewise for the square rectangle

$$F_R = (\gamma h/2) (l_2 h) = \frac{\gamma l_2 h^2}{2}$$

The total force acting on the dam is then

$$F = 2F_T + F_R = \gamma h^2 \left(\frac{l_1}{3} + \frac{l_2}{2} \right)$$

$$F = (62.428)(715)^2 \left(\frac{295}{3} + \frac{290}{2} \right)$$

$$\boxed{F = 7.7 \times 10^8 \text{ lb}}$$

The points of action will be along the axis of symmetry. To compute the height on this axis, weight the centroids by the force on each component:

$$y_c = \frac{2C_T F_T + C_R F_R}{F_T + F_R}$$

$$y_c = h \left[\frac{4l_1 + 9l_2}{12l_1 + 18l_2} \right]$$

$$y_c = 0.432h$$

To get the answer in the book, we need to put the origin at the base so

$$y_c = (1 - 0.432)h = 406 \text{ ft from the base.}$$

Method 2: Instead of using the centroids, the force on a part of the dam can be computed by integrating the pressure over the area:

$$F_i = \int_0^h \underset{\substack{\uparrow \text{width of the section as a function of } y}}{w_i(y)} \underset{\substack{\uparrow \text{pressure}}}{\gamma y} dy$$

For the triangle,

$$w_T = l_1 (1 - y/h)$$

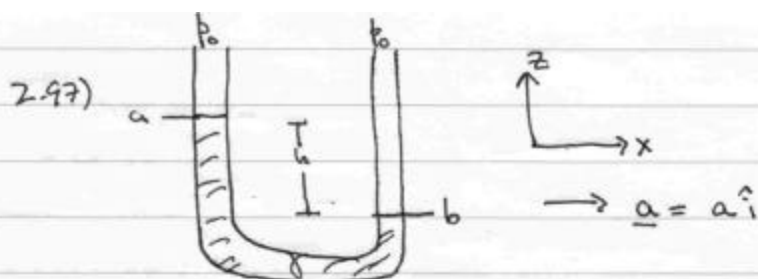
$$F_T = \int_0^h l_1 (1 - \frac{y}{h}) \gamma y dy = \frac{l_1 \gamma h^2}{6}$$

For the square, rectangle

$$w_R = l_2$$

$$F_R = \int_0^h l_2 \gamma y dy = \frac{l_2 \gamma h^2}{2}$$

which of course agree with the results of the centroid method.



From the hydrostatic eqn

$$-\nabla p = \gamma \hat{k} + \rho a \hat{i}$$

we have the components

$$\frac{\partial p}{\partial z} = -\gamma \quad \text{and} \quad \frac{\partial p}{\partial x} = \rho a$$

Integrate both eqns

$$p(x, z) = -\gamma z + f(x)$$

$$p(x, z) = \rho a x + g(z)$$

These are satisfied by the choice

$$f(x) = \rho a x + C$$

$$g(z) = -\gamma z + C$$

So the pressure is

$$p(x, z) = -\gamma z + \rho a x + C$$

Since both a and b are open to the atmosphere

$$p_a = p_b$$

$$-\gamma z_a + \rho a x_a + C = -\gamma z_b + \rho a x_b + C$$

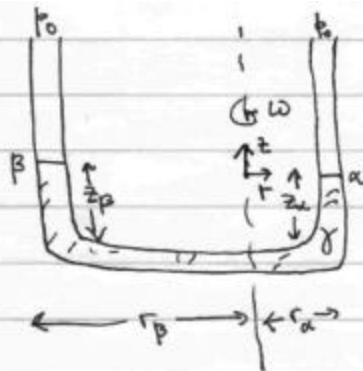
$$\gamma(z_a - z_b) + \rho a(x_a - x_b) = 0$$

$$\gamma h + \rho a(-l) = 0$$

So the relationship between a, l, and h is

$$\boxed{\gamma = \frac{\rho a l}{h}} \quad \text{or} \quad \boxed{g = \frac{a l}{h}}$$

2-100)



We are given $r_\alpha = 90\text{mm}$; $r_\beta = 220\text{mm}$
and $|z_\beta - z_\alpha| = 75\text{mm}$.

From eqn (2-33) in Munson et. al,

$$p(r, z) = \frac{\rho \omega^2 r^2}{2} - \gamma z + C$$

Since the position α and β are open to the air:

$$p_\alpha = p_\beta$$

Using the expression for the pressure

$$\frac{\rho \omega^2}{2} (r_\alpha^2 - r_\beta^2) - \gamma (z_\alpha - z_\beta) = 0$$

$$\omega = \frac{2\gamma (z_\alpha - z_\beta)}{\rho (r_\alpha^2 - r_\beta^2)}$$

Since $r_\alpha^2 - r_\beta^2 < 0$, then we know $z_\alpha - z_\beta < 0$ and the point β is higher than the point α . Noting that $\gamma = \rho g$

$$\omega = \frac{2g (z_\alpha - z_\beta)}{r_\alpha^2 - r_\beta^2}$$

$$\omega = \frac{2(9.8 \text{ m/s}^2)(-75 \text{ mm})(1 \text{ m}/1000 \text{ mm})}{\{(90 \text{ mm})^2 - (220 \text{ mm})^2\}(1 \text{ m}/1000 \text{ mm})^2}$$

$$\boxed{\omega = 6.03 \text{ rad}}$$