

Professor Michael Triantafyllou; Professor Kim Vandiver for Shear7 and mechanical vibration consultation; Dr. Dave Burke and Yingbin Bao for structures and Abaqus instruction; Micaela Pilotto for Abaqus instruction and consultation; the MIT Department of Ocean Engineering for supporting the design team; and our contacts in industry: Peter Young of Shell Exploration and Production Company; Dr. Steve Leverette of Atlantia Offshore, Ltd; and Chad Musser.

The first and most important part of the process is to obtain quality current profiles for varying currents, in order to see the dynamic response of the tendons in the largest cross-section of environments possible. Eight current profiles were used in our analyses. The current profiles (the 100 year storm, reduced extreme storm, OTC 8405, Typhoon and Non Typhoon) are published proceedings. After obtaining a sufficient array of current conditions, the next step is to prepare the VIVA requires a set of input files which describe the physical and material properties of the tendons, Once the input files are properly generated they can be fed to VIVA which then produces an extensive set of output files. The first set of results is the overall motion, first with the separate modal responses graphed independently and then the full spectrum response.

# TENSION LEG PLATFORM DESIGN: **OPTIMIZATION FOR VORTEX INDUCED VIBRATION** MEG BROGAN and KATIE WASSERMAN, Massachusetts Institute of Technology Tension Leg Platform design is a challenging and popular area of research in the offshore oil industry. In order to compete in the

International Student Offshore Design Competition (ISODC), a Tension Leg Platform (TLP) was designed. Our TLP design addresses five fundamental areas of technical competency (General Arrangement and Overall Hull/System Design, Weight, Buoyancy and Stability, Global Loading, General Strength and Structural Design, Risk Assessment) and three specialized areas of technical competency unique to a Vortex Induced Vibration (VIV) optimized design (Hydrodynamics of Motions and Loading,

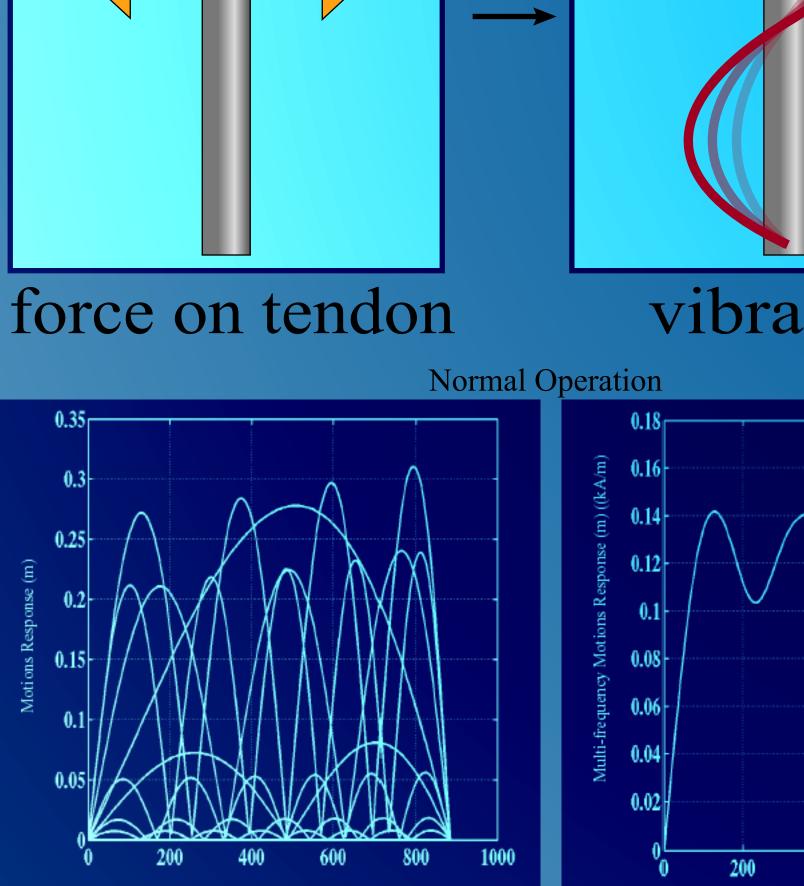
Our design optimization process begins with a four-caisson, four-pontoon tension leg platform, operating at a depth of 3,000 ft. Hydrostatic and hydrodynamic analysis for design iterations are performed by our own MATLAB script, which calculates the effects of motions due to Vortex Induced Vibration (VIV). Structural analysis addresses fatigue loading from VIV. Our design includes risk-based analysis and conforms to class society rules and regulations. VIV phenomena cause uncontrollable motions of offshore platforms, as well as fatigue damage and failure of components such as cables and risers. The effects of VIV need to be addressed early in the design process to avoid costly platform damage and costly retrofits, such as hydrodynamic strakes for platform tendons.

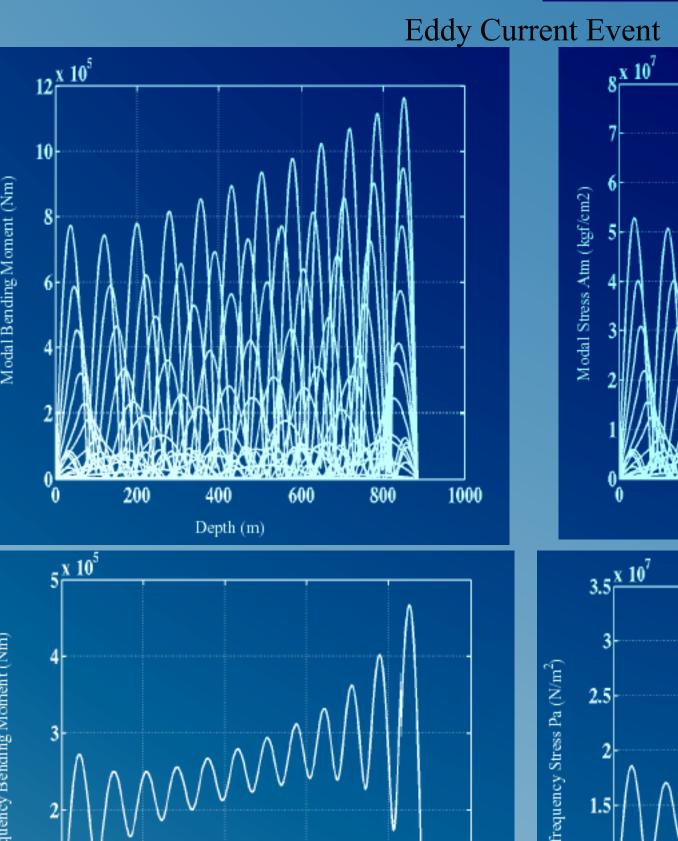
# SPECIALIZED DESIGN

### FINTE ELEMENT ANALYSIS

In order to calculate displacements and loads on the structure, it is common practice to use a commercial finite element software program, such as Abaqus. In finite element analysis, as described by Thomas J. R. Hughes, a continuous structure such as a plate or beam is divided into discrete elements, and continuous loads are divided into discrete nodal point loads. The elements are connected at nodes. The most common elements are triangular and rectangular elements. Elements can be the same size throughout the structure, or a "graded mesh" where the elements are smaller in the region where a more detailed modeling is desired. The advantage of triangular elements is a constant stress value within the element. Finite element analysis always predicts deflections that are less than the deflections predicted by elastic beam theory. To satisfy compatibility, a displacement function is assumed, which causes the finite element model to be stiffer than the actual structure.

## **VORTEX INDUCED VIBRATION**



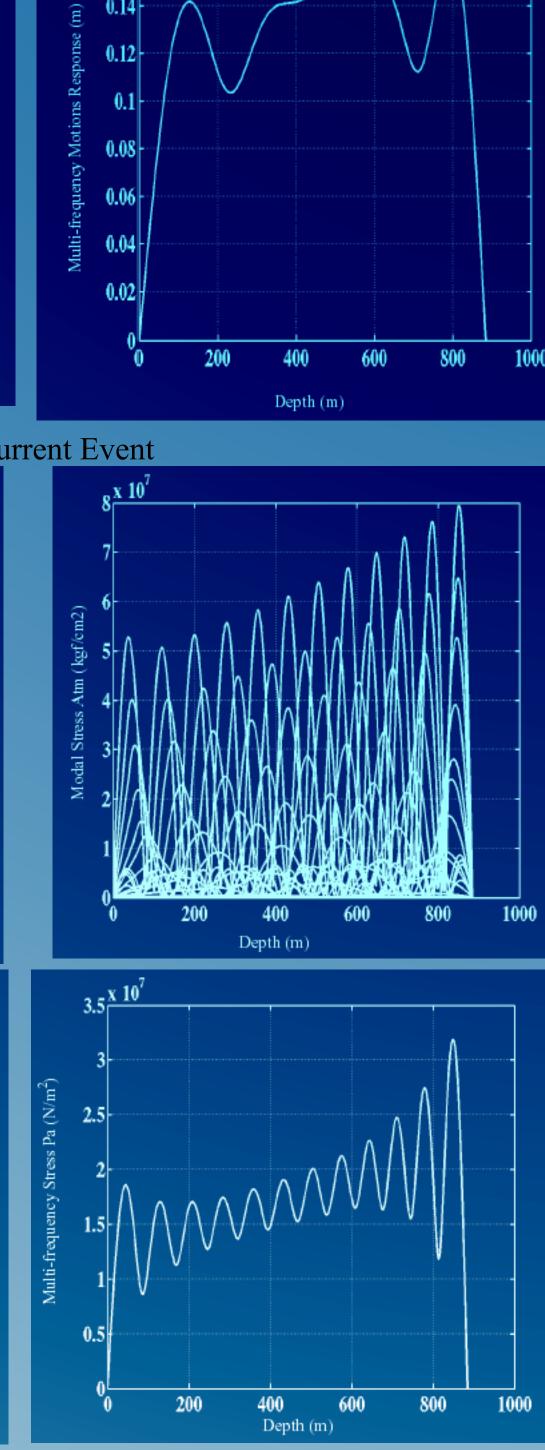


0 200 400 600 800

Depth (m)

Depth (m)





# FUNDAMENTAL DESIGN

### WEIGHT, BOUYANCY, STABLITY

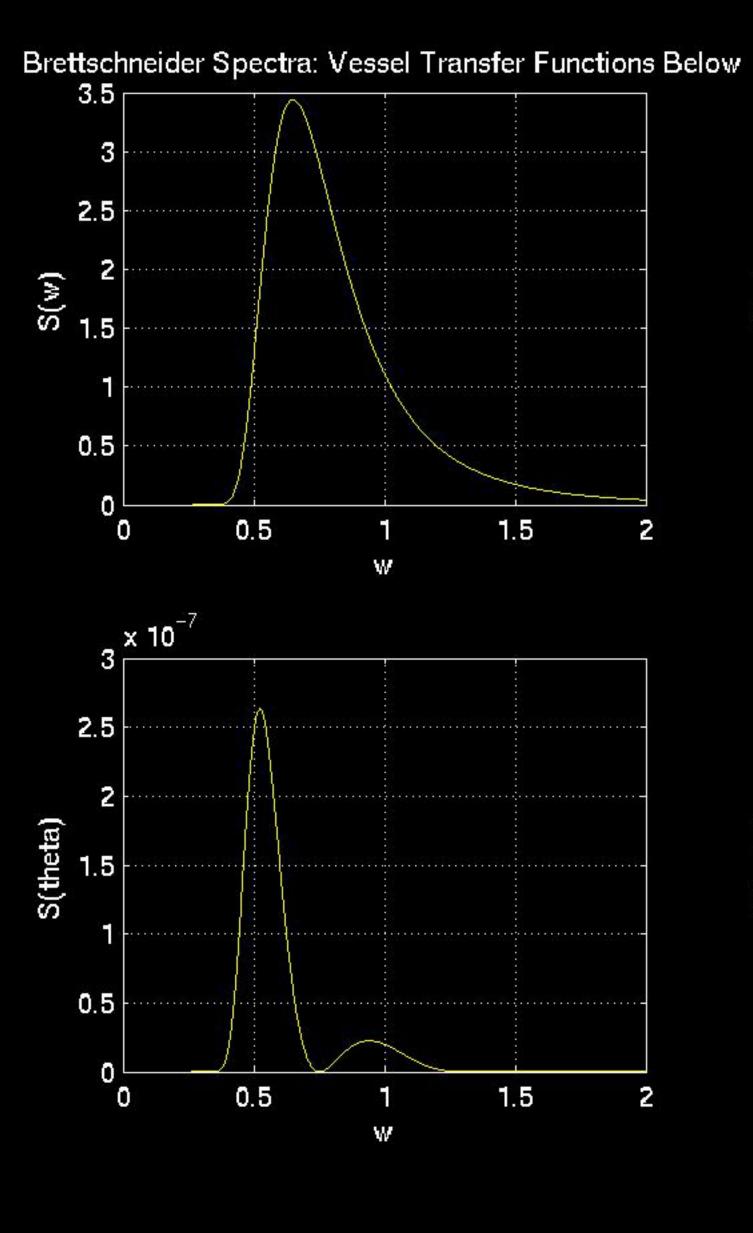
	W	<b>veights</b>											
total topside weight				20502 tons									Th
hull weight				12054 tons			System Forces an						
tendon weight				7500 tons			total hull displacement				51230 tons of		
total vessel weight				36306 tons			total buoyancy						con cai
total weight with 15% margin				41752 tons			tendon tension				790 tons a		and
Natural Frequency				Natural I			eriod Effectiv			ive S	ve Stiffness the		
heave				heave		1.9  rad/s		]	neave	1.54 x			
pitch	23.11 s		ľ	pitch 0.		271	2719 rad/s		pitch	5.10 x	$10^{10}$	lbs/ft	ten
roll	23.11 s			roll	roll 0.2		2719 rad/s		roll	5.10 x	$10^{10}$	lbs/ft	are are
Displacements					۵dde	h	Mass						stif yie
		740 tons		pontoons		116010 slugs		lgs					tog
	pontoons 1665			caisso			<u> </u>					loc she	
tendons	tendons 83		tota		tal 1773000 slı		ugs					5110	
Radius of Gyration					Μ	[01	nents of	Ine	ertia				
superstructure 91.88			ft	superstructure				5.92 x 10	) <sup>9</sup> ft4				
deck		61.25	61.25 ft		deck		eck		2.63 x 10	) <sup>9</sup> ft 4			
caissons		33.25 ft			caissons			1.67 x 10			ATI	LP is a con	
exposed	44.63	44.63 ft		exposed				7.42 x 10		t <sup>4</sup> concept. Ur		ept. Unlil	
sheilded	16.62	5.62 ft		sheilded por		pontoons	oons 1.03 x 10 <sup>1</sup>		$)^{10} {\rm ft}^4$			orms resp LP is comp	
<b>Operating Environment</b>				XX/	otorn	21	$he \Lambda rea =$	= 12	202 ft			surge	e and sway
sea state		5	5		Waterplane Area = $13893 \text{ ft}_2$					TLP is fixed. TLP is fixed.			
Н		5 ft											use the $b_1$
natural frequency		0.65 ra	d/s										cal moorii cally stabl

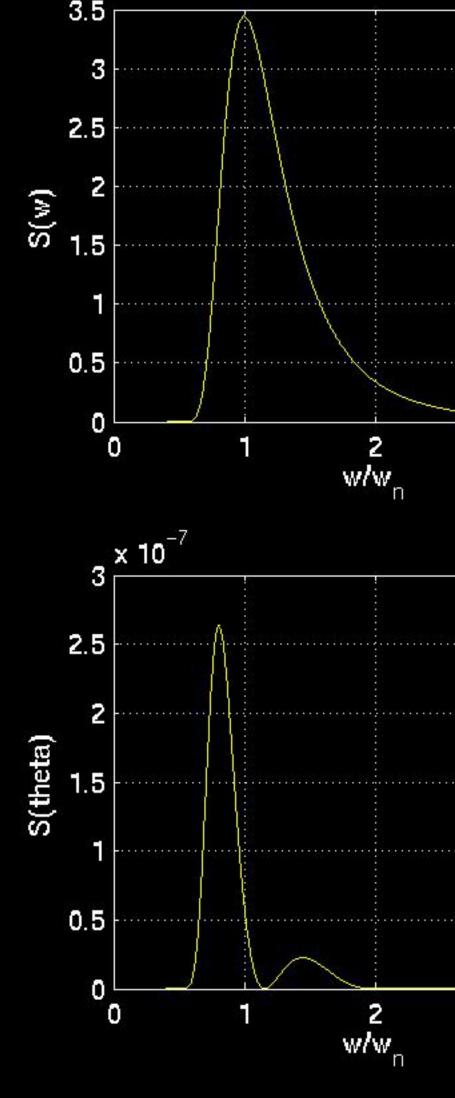
Fatigue Life

lurrent	Multi- Modal Minimum Fatigue Life (years)	Multi- Modal Location (m)	Modal Minimum Fatigue Life (years)
10 Year Storm	5100	833.7	43
Eddy Surrent Event educed	63	851.5	.22
streme Storm	68000	821.9	810
OTC 8045	1600	833.7	17
OTC 8606	240	848.5	1.5
yphoon	6.6	851.5	.052
Non- yphoon	43000	836.7	370
urrent	Modal Location (m)	Problem Mode	Associated Stress Pa (N/m <sup>2</sup> )
10 Year Storm	833.7	7	8 x 10 <sup>6</sup>
Eddy Surrent Event	851.5	12	2 x 10 <sup>7</sup>
educed xtreme Storm	824.9	6	4.6 x 10 <sup>6</sup>
educed xtreme	824.9 833.7	6 7	4.6 x 10 <sup>6</sup> 1 x 10 <sup>7</sup>
educed xtreme Storm OTC			
educed xtreme Storm OTC 8045 OTC	833.7	7	1 x 10 <sup>7</sup>
educed xtreme Storm OTC 8045 OTC 8606	833.7 848.5	7	1 x 10 <sup>7</sup> 1.4 x 10 <sup>7</sup>

and Multi-Modal Bending Moments and Stresses for Eddy Current Event Condition						
ent	Multi- modal Bending Moment (Nm)	Multi- modal Stress Pa (N/m2)	Modal Bending Moment (Nm)	Modal Stress Atm (kgf/cm2)		
nal ation	6.5 x 104	4.3 x 106	1.3 x 105	8.7 x 106		
Year n	2 x 105	13.5 x 106	4.2 x 105	28 x 106		
ent it	4.7 x 105	32 x 106	10.8 x 105	80 x 106		
iced eme n	1.1 x 105	7.5 x 106	2.2 x 105	14.9 x 106		
8045	2.4 x 105	17 x 106	5 x 105	34 x 106		
8606	3.5 x 105	24 x 106	7.6 x 105	53 x 106		
loon	7 x 105	52 x 106	16 x 105	110 x 106		
loon	1.13 x 105	7.7 x 106	2.5 x 105	17 x 106		

### Hydrodynamic Analysis





in the Gulf of Mexico.

### RISK ASSESSMENT

Determining risks and managing risks are two separate processes, once aware of your potential hazards, it is imperative that offshore engineer has a system which monitors the vessel operations so as to warn against impending problems. To ensure that a vessel, TLP in our case, is performing satisfactorily during operation, operators make use of barrier diagrams, Bow-Tie analysis and criticality reviews. Bow-tie analyses are where one connects a primary event with its potential consequences, threats, preventative measures and recovery measures. The operator must monitor the mechanical integrity of the vessel as well as the SHE (safety, health and environment) systems. Control measures, to prevent occurrences or mitigate problems, are linked to something called a platform SMS (safety management system). Most all operating platforms have one of these systems, in one form or another, and through them, they manage the key barriers to failure and the performance standards of the vessel.

GLOBAL LOADING, STRENGTH AND STRUCTURAL DESIGN

he global loads on the structure are weight, buoyancy, nd wave and current loading. The structural components The TLP are made of steel. The critical structural ponents of a TLP are the tendons, foundations, sons and pontoons, connections between columns d pontoons, deck girders, and connections between e deck and pontoons. Because they are long columns, tendons are subject to buckling. Tendon pre-tension is static, permanent load on the TLP foundations. Environmental loads such wave loads and currents are variable loads, and lateral inclination of the ndons causes lateral loads on the foundations. The TLP caissons and pontoons e orthogonally stiffened shells. The caisson shells have a cylindrical oss-section and the pontoon shells have a rectangular cross section. The ffened shells are subject to buckling failure under compressive loads and elding under tensile loads. The stringers and attached shell plate may buckle ether, the panels themselves may buckle, or the shell plating may buckle cally, while the stiffeners remain stable. The deck girders, like the stiffened

Ils, may buckle or yield, but are not subject to external water pressure.

### GENERAL ARRANGEMENT OVERALL HULL/SYSTEM DESIGN

pliant, free-floating offshore platform ike fixed offshore platforms, compliant ond to external effects with motions. liant in the horizontal degrees of freedom, y. In the vertical degrees of freedom, a The feature that distinguishes a TLP from platform concepts is its reserve buoyancy. uoyancy of a TLP exceeds its weight, ngs called "tendons" keep the TLP vertically stable and control heave motions. Our TLP design is based on Shell's Brutus TLP

water depth hull breadth

draft caisson diameter caisson height pontoon width pontoon height tendon diameter tendon wall thickness number of tendons

254 ft. 85 ft. 66.5 ft. 166 ft. 35.5 ft. 23 ft. 2.6667 1.25 ft.

2985 ft.

superstructure

deck

caissons

pontoons

tendons

pilings