# Unit M4.6 <br> Torsion of Rods/Shafts 

## Readings:

CDL 6.1-6.5
16.003/004 -- "Unified Engineering"

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## LEARNING OBJECTIVES FOR UNIT M4.6

Through participation in the lectures, recitations, and work associated with Unit M4.6, it is intended that you will be able to..........

- ....describe the key aspects composing the model of a (torsional) shaft and identify the associated limitations
- ....apply the basic equations of elasticity to derive the solution for the general case
- ....identify the parameters that characterize torsional behavior and describe their role

Thus far we've considered a long slender member under axial load (rod) and bending load (beam). Let's now look at a long slender member subjected to a torque. This is a shaft.

Let's begin with the...

## Definition of a Shaft

A shaft is a structural member that is long and slender and subjected to a torque moment about its long axis.

Consider each of the three points that make up the definition and the true reality...
--> Modeling Assumptions
a) Geometry
(go back to indicial notation because it makes it easier to manipulate)

Figure M4.6-1 General geometry of shaft


Figure M4.6-2 Cross-section of shaft

$\mathrm{b}, \mathrm{h}=$ largest dimensions in $x_{2}$ and $x_{3}$

Assumption: "long" in $\mathrm{x}_{1}$-direction
$\mathrm{L} \gg \mathrm{b}, \mathrm{h}$ (slender member)
Note: same as before
b) Loading

Assumption: Torque Moment about $\mathrm{x}_{1}$-direction

- concentrated T

- distributed


No axial loads
$\Rightarrow$ at boundaries: $\sigma_{11}=\sigma_{22}=\sigma_{33}=0$
finally look at:
c) Deformation

Assumptions:

- At any location $x_{1}$, the cross-section rotates as a rigid body ( $\Rightarrow$ no distortion of cross-section)

Note: can also say "plane sections remain plane and perpendicular to midline"

- No deformation of cross-section in $\mathrm{x}_{1}$-direction (no bending or extension)
$\Rightarrow$ Only deformation is rotation of cross-section through a twist angle.
Define twist angle, $\phi$, as function of $\mathrm{x}_{1}=\phi\left(\mathrm{x}_{1}\right)$. Think of deck of cards:


We can, by geometry, relate the deformations, $u_{i}$, to the twist/rotation angle $\phi\left(\mathrm{x}_{1}\right)$.

Consider a cross-section at location $x_{1}$, and a point in a circular crosssection at angle $\beta$ from the reference axes.
Figure M4.6-2 Illustration of deformation of shaft cross-section


- Distance point $p$ rotates $(p$ to $P)=r \sin \phi\left(x_{1}\right)$
- For small angles (assumed here)

$$
\Rightarrow \text { distance }=r \phi\left(x_{1}\right)
$$

Resolve into components along $x_{2}$ and $x_{3}$ Figure M4.6-3 Resolution of deformation into components along $x_{2}$ and $x_{3}$


$$
\Rightarrow u_{2}=-r \phi\left(x_{1}\right) \sin \beta
$$

note direction

$$
u_{3}=+r \phi\left(x_{1}\right) \cos \beta
$$

And $\beta$ is defined by the $x_{2}$ and $x_{3}$ location of $p$


$$
\begin{aligned}
& r=\sqrt{x_{2}^{2}+x_{3}^{2}} \\
& \sin \beta=\frac{x_{3}}{r} \\
& \cos \beta=\frac{x_{2}}{r}
\end{aligned}
$$

So:

$$
\begin{aligned}
& u_{2}=-\sqrt{x_{2}^{2}+x_{3}^{2}} \phi\left(x_{1}\right) \frac{x_{3}}{\sqrt{x_{2}^{2}+x_{3}^{2}}}=-\phi\left(x_{1}\right) x_{3} \\
& u_{3}=\sqrt{x_{2}^{2}+x_{3}^{2}} \phi\left(x_{1}\right) \frac{x_{2}}{\sqrt{x_{2}^{2}+x_{3}^{2}}}=\phi\left(x_{1}\right) x_{2}
\end{aligned}
$$

And we have, by assumption, no axial displacement. So the assumed displacement state is:

$$
\begin{align*}
& u_{1}=0  \tag{1}\\
& u_{2}=-\phi\left(x_{1}\right) x_{3}  \tag{2}\\
& u_{3}=\phi\left(x_{1}\right) x_{2} \tag{3}
\end{align*}
$$

Let's now use the definitions in the equations of elasticity to get the...

## Governing Equations

--> Strain-Displacement

$$
\left.\begin{array}{l}
\left.\varepsilon_{11}=\frac{\partial u_{1}}{\partial x_{1}}=0\right\} \\
\varepsilon_{22}=\frac{\partial u_{2}}{\partial x_{2}}=0 \\
\varepsilon_{33}=\frac{\partial u_{3}}{\partial x_{3}}=0
\end{array}\right\} \begin{aligned}
& \text { consistent with assumption that cross- } \\
& \text { section does not deform in } \mathrm{x}
\end{aligned} \begin{aligned}
& \text { consistent with assumption that cross- } \\
& \text { section does not distort } \tag{5}
\end{aligned}
$$

$$
\begin{aligned}
& \text { Note: } \frac{\partial}{\partial x_{1}} \rightarrow \frac{d}{d x_{1}} \text { since } \phi \text { is a function of } \mathrm{x}_{1} \text { only } \\
& \text { (partial) } \\
& \text { (total) }
\end{aligned}
$$

Finally: $\varepsilon_{23}=\frac{1}{2}\left(\frac{\partial u_{2}}{\partial x_{3}}+\frac{\partial u_{3}}{\partial x_{2}}\right)=\frac{1}{2}\left(-\phi\left(x_{1}\right)+\phi\left(x_{1}\right)\right)=0$
$\Rightarrow$ also is consistent with assumption that cross-section does not distort

Next go to...
--> Stress-Strain Equations
(do for isotropic $\Rightarrow$ only one shear modulus)
Since $\varepsilon_{11}, \varepsilon_{22}$, and $\varepsilon_{33}=0 \quad \Rightarrow \sigma_{11}, \sigma_{22}, \sigma_{33}=0$
(consistent: no axial stresses)

$$
\begin{gather*}
\varepsilon_{23}=\frac{\sigma_{23}}{2 G}=0 \Rightarrow \sigma_{23}=0 \\
\varepsilon_{12}=\frac{\sigma_{12}}{2 G}  \tag{6}\\
\varepsilon_{13}=\frac{\sigma_{13}}{2 G}  \tag{7}\\
\Rightarrow \text { only } \sigma_{12} \text { and } \sigma_{13} \text { are present }
\end{gather*}
$$

Finally we look at...
--> Equilibrium Equations
First we again define an internal stress resultant for the structural configuration. In this case, it will be the torque moment at any point.

Cutting the shaft......
Figure M4.6-4 Illustration of cutting shaft through cross-section and considering internal torque


+ right hand rule -- gives equal and opposite

Express $\mathbf{T}$ in terms of the stress:

Figure M4.6-5 Illustration of equipollence consideration for shaft crosssection

equivalence/equipollence:

$$
\begin{equation*}
\Rightarrow \text { Torque: } T=\iint\left(x_{2} \sigma_{13}-x_{3} \sigma_{12}\right) d x_{2} d x_{3} \tag{8}
\end{equation*}
$$

So using equations of equilibrium (considering only non zero stresses)

$$
\begin{align*}
& \frac{\partial \sigma_{21}}{\partial x_{2}}+\frac{\partial \sigma_{31}}{\partial x_{3}}+X_{0}=0 \\
& \frac{\partial \sigma_{12}}{\partial x_{1}}+f_{2}=0  \tag{10}\\
& \frac{\partial \sigma_{13}}{\partial x_{1}}+f_{3}=0 \tag{11}
\end{align*}
$$

Now look at the equilibrium of a discrete segment (as we have in the past)

Figure M4.6-6 Geometry for consideration of equilibrium of a discrete segment



$$
\begin{align*}
& \sum M_{x_{1}}=0 \stackrel{+}{\leftrightarrows} \Rightarrow-T+t\left(x_{1}\right) d x_{1}+T+\frac{d T}{d x_{1}} d x_{1}=0 \\
& \Rightarrow \frac{d T}{d x_{1}}=-t\left(x_{1}\right) \tag{12}
\end{align*}
$$

## "Torque-loading Relation"

$$
\left(\text { like } \frac{d S}{d x}=q(x)\right)
$$

Note: Can show average equilibrium relations [resulting in (12)] are consistent with pointwise relations [(9) - (11)].

We now have 6 unknowns ( $T, \sigma_{12}, \sigma_{13}, \varepsilon_{12}, \varepsilon_{13}, \phi$ ) and 6 equations $\{(4)$, (5), (6), (7), (8), (12) \}. This allows us to solve the problem for this model.

So let's look at.....

## Solution and Limitations of Model

Put equations (4) and (5) [Strain-Displacement] into the stress-strain equations (6) and (7):

$$
\begin{align*}
& \sigma_{12}=2 G \varepsilon_{12}=2 G\left(-\frac{1}{2} x_{3} \frac{d \phi}{d x_{1}}\right) \\
& \Rightarrow \sigma_{12}=-G x_{3} \frac{d \phi}{d x_{1}}  \tag{13}\\
& \sigma_{13}=2 G \varepsilon_{13}=2 G\left(\frac{1}{2} x_{2} \frac{d \phi}{d x_{1}}\right) \\
& \Rightarrow \sigma_{13}=G x_{2} \frac{d \phi}{d x_{1}} \tag{14}
\end{align*}
$$

Now place these results into the torque-stress equilibrium equation (8):

$$
\begin{aligned}
T & =\iint\left(x_{2} \sigma_{13}-x_{3} \sigma_{12}\right) d x_{2} d x_{3} \\
= & \iint G\left(x_{2}^{2} \frac{d \phi}{d x_{1}}+x_{3}^{2} \frac{d \phi}{d x_{1}}\right) d x_{2} d x_{3} \\
& \Rightarrow T=G \frac{d \phi}{d x_{1}} \iint\left(x_{2}^{2}+x_{3}^{2}\right) d A
\end{aligned}
$$

Define:

$$
\begin{equation*}
J \equiv \iint\left(x_{2}^{2}+x_{3}^{2}\right) d A \tag{15}
\end{equation*}
$$

> = polar (second) moment of inertia

$$
=\frac{\pi R^{4}}{2} \quad \text { for circle }
$$

So we write:

$$
\begin{equation*}
T=G J \frac{d \phi}{d x_{1}} \tag{16}
\end{equation*}
$$

"Torque-Twist" relation

Note, again, overall "structural constitutive relation"

$$
\begin{aligned}
T & =G J \frac{d \phi}{d x_{1}} \\
(\mathrm{load}) & =(\text { stiffness })(\text { deformation })
\end{aligned}
$$

Structural stiffness here is torsional stiffness $=G J$
composed of two parts:
G - material contribution/parameter
J-geometrical contribution/parameter
similar to bending:

$$
M=E_{x} I \frac{d^{2} w}{d x^{2}}
$$

We can use the result $G \frac{d \phi}{d x_{1}}=\frac{\mathrm{T}}{\mathrm{J}}$ in equations (13) and (14) to
relate stress to torque:

$$
\begin{align*}
\sigma_{12} & =-\frac{T x_{3}}{J}  \tag{17}\\
\sigma_{13} & =\frac{T x_{2}}{J} \tag{18}
\end{align*}
$$

Finally, can express the stress as a shear stress resultant:
Figure M4.6-7 Illustration of shear stress resultant


$$
\begin{equation*}
\tau_{\text {res }}=\frac{T r}{J} \tag{19}
\end{equation*}
$$

Note similarity to bending:

$$
\begin{aligned}
& \sigma_{x x}=-\frac{M z}{I} \\
& \text { form: } \\
& (\text { stress })=\frac{\binom{\text { primary }}{\text { loading }}\binom{\text { distance from }}{\text { key point }}}{\binom{\text { geometrical }}{\text { parameter }}}
\end{aligned}
$$

## Use of Model

Very similar to rod, beam...

1. Draw Free Body Diagram, determine reactions
2. Get internal stress resultant $T\left(x_{1}\right)$
3. Determine section property $G J$ material
4. Use equation (16) to find rate of twist $\frac{d \phi}{d x_{1}}$
polar moment of inertia
5. Use equations (17), (18), and (19) to determine stresses
6. Determine strains and displacements as needed
let's think about the...
--> Limitations of the model
The assumptions give us an exact solution for circular closed crosssections:
Solid

$$
J=\frac{\pi R^{4}}{2}
$$

Tube


$$
J=\frac{\pi R_{o}^{4}}{2}-\frac{\pi R_{i}^{4}}{2}
$$

by superposition

Not good for an open section, for example...


Approximate for other closed sections:
e.g., Square


$$
J=0.141 \mathrm{a}^{4}
$$

(assumption of no deformation of cross-section violated --> "warping") [more in 16.20]

We'll next look at a rod under compression and look at an instability phenomenon known as "buckling". In this case, we call the structural member a column.

## Unit M4.6 (New) Nomenclature

G -- shear modulus (isotropic material)
GJ -- torsional stiffness
J -- polar (second) moment of inertia
$\mathrm{R}_{\mathrm{o}}$-- outer radius
$\mathrm{R}_{\mathrm{i}}$-- inner radius
T -- applied point torque load
t -- applied distributed torque load
$\tau_{\text {res }}$-- shear stress resultant
$\phi-$ twist angle

