

3.11 Solutions Problem Set # 6

Problem #1 Determine the maximum shear stress and rate of twist of the given shaft if a 10 kNm torque is applied to it. If the length of the shaft is 15 m, how much would it rotate by? Let $G = 81 \text{ GPa}$, $D = 75 \text{ mm}$

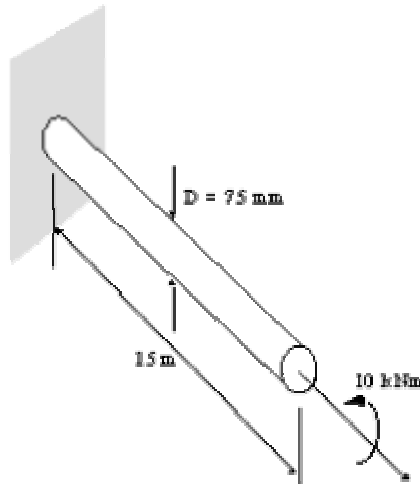


Figure 90: Shaft of example 13, subject to 10 kNm torque

$$J = \frac{\pi D^4}{32} = \frac{\pi (0.075)^4}{32} = 3.106 \times 10^{-6} \text{ m}^4$$

$$\tau = \frac{Tr}{J} = \frac{10 \times 10^3 \times 0.0375}{3.1063 \times 10^{-6}} = 120.7 \text{ MPa}$$

$$\frac{d\theta}{dx} = \frac{T}{GJ} = \frac{10 \times 10^3}{81 \times 10^9 \times 3.1063 \times 10^{-6}} = 0.03974 \text{ rad/m}$$

Which equates to :

$$\frac{d\theta}{dx} = 2.277 \text{ }^\circ/\text{m}$$

If the shaft is 15 m long, the angle of rotation at the free end is 34.157° degrees.

Problem #2 GERE Problem 3.3-3

Aluminum Bar in Torsion

$$a) k_T = G I_p / L = G \pi d^4 / 32 L = (3.8 \text{e6 psi})(\pi)(1.0 \text{in})^4 / (32 * 48 \text{in}) = 7770 \text{lb-in}$$

$$b) \Phi = 5^\circ = 5 * \pi / 180 \text{ rad} = 0.087 \text{rad}$$

$$\Phi = T L / G I_p \rightarrow T = G I_p \Phi / L$$

$$\tau_{\text{max}} = T r / I_p = T d / 2 I_p \rightarrow \text{plug in } T, \text{ so}$$

$$\tau_{\text{max}} = G d \Phi / 2 L = 3450 \text{psi}$$

Max Shear Strain

$$\text{Hooke's law: so } \gamma_{\text{max}} = \tau_{\text{max}} / G = 909 \text{e-6 rad}$$

Problem #3 GERE Problem 3.4-2

Polar Moments of Inertia for AB, BC and CD

$$\text{Calc } I = \pi / 32 * d^4$$

$$\text{AB: } I = 4.021 \text{e6 mm}^4$$

$$\text{BC: } I = 1.272 \text{e6}$$

$$\text{CD: } I = 0.2513 \text{e6}$$

AB

a) Shear Stresses

$$\text{AB: } \tau = T_{AB} r_{AB} / I_{pAB} = 57.7 \text{ MPa}$$

$$\text{BC: } \tau = T_{BC} r_{BC} / I_{pBC} = 66.0 \text{ MPa}$$

$$\text{CD: } \tau = T_{CD} r_{CD} / I_{pCD} = 63.7 \text{ MPa}$$

$$\tau_{\text{max}} = 66.0 \text{Mpa}$$

b) angle of twist

$$\text{AB: } \Phi_{AB} = T_{AB} L_{AB} / G (I_p)_{AB} = 0.00902 \text{rad}$$

$$\text{BC: } \Phi_{BC} = T_{BC} L_{BC} / G (I_p)_{BC} = 0.01376 \text{rad}$$

$$\text{CD: } \Phi_{CD} = T_{CD} L_{CD} / G (I_p)_{CD} = 0.01990 \text{rad}$$

$$\Phi_D = \Phi_{AB} + \Phi_{BC} + \Phi_{CD} = 0.04268 \text{ rad} = 2.45 \text{degrees}$$

Stresses and Strains within a Beam**Problem #4 GERE Problem 5.5-3**

a) Maximum Bending Stress

$$\alpha * \rho = L \text{ so } \rho = L / \alpha$$

$$\sigma_{\text{max}} = E y / \rho = E (t/2) \alpha / L = 50,600 \text{psi}$$

b) Change in stress

If the angle α increases, the stress σ_{max} increases.**Problem #5 GERE Problem 5.5-6**

$$M_{\text{max}} = P b$$

$$S = \pi / 32 * d^3$$

$$\sigma_{\text{max}} = M_{\text{max}} / S = 32 P b / (\pi d^3) = 185.0 \text{MPa}$$

Problem #6 Horizontal Shear Stresses

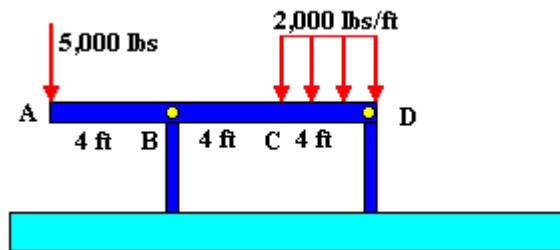
STATICS & STRENGTH OF MATERIALS

A loaded, simply supported W 10 x 45 beam is shown below. For this beam:

A. Determine the maximum bending stress 6 feet from the left end of the beam.

B. Determine the horizontal shear stress at a point 4 inches above the bottom of the beam cross section and 6 feet from the left end of the beam.

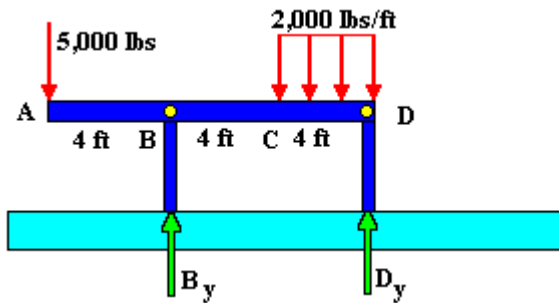
Unless otherwise indicated, all joints and support points are assumed to be pinned or hinged joints.



Solution:

Part A:

STEP 1: Determine the external support reactions:



- 1.) FBD of structure (See Diagram)
- 2.) Resolve all forces into x/y components
- 3.) Apply equilibrium conditions:

$$\sum F_x = 0 \text{ none}$$

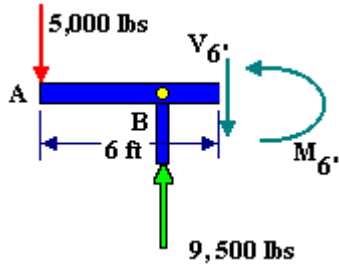
$$\sum F_y = B_y + D_y - 2,000 \text{ lbs/ft} (4 \text{ ft}) - 5,000 \text{ lbs} = 0$$

$$\sum T_B = 5,000 \text{ lbs} (4 \text{ ft}) - 2,000 \text{ lbs/ft} (4 \text{ ft}) (6 \text{ ft}) + D_y(8 \text{ ft}) = 0$$

Solving: $B_y = 9,500 \text{ lbs}$; $D_y = 3,500 \text{ lbs}$

STEP 2: Determine the shear force and bending moment at $x=6 \text{ ft}$.

- 1.) Cut beam at 6 ft. Draw the FBD of left end of beam, showing and labeling all external forces.



- 2.) Resolve all forces into x/y directions.
 3.) Apply equilibrium conditions:

$$\sum F_x = 0 \text{ none}$$

$$\sum F_y = -5,000 \text{ lbs} + 9,500 \text{ lbs} - V_6 = 0$$

$$\sum T_A = 9,500 \text{ lbs} (4 \text{ ft}) - 4,500 \text{ lbs} (6 \text{ ft}) + M_6 = 0$$

$$\text{Solving: } V_6 = 4,500 \text{ lbs; } M_6 = -11,000 \text{ ft-lbs}$$

STEP 3: Apply the Flexure Formula to determine the Maximum Bending Stress (MBS) at 6'.

$MBS = M_6/S$ (Where M_6 is the bending moment at 6 ft, and S is the section modulus for the beam. The section modulus is available from the Beam Tables. The W 10 x 45 beam has a section modulus for the beam from the beam tables is 49.1 in^3 .)

$$MBS = -11,000 \text{ ft-lbs}(12 \text{ in/ft})/49.1 \text{ in}^3 = -2,688 \text{ lbs/in}^2$$

Part B:

STEP 4: To determine the Horizontal Shear Stress (HSS) at 6 ft from the end of the beam and 4 inches above the bottom of the beam, apply the horizontal shear stress formula.

The form we will use is: $HSS = Vay'/Ib$

Where:

V = Shear force 6 ft from the end of the beam

a = cross sectional area from 4 in above the bottom of the beam to bottom of beam

y' = distance from neutral axis to the centroid of area a

I = moment of inertia of the beam (249 in^4 for W 10 x 45 beam)

b = width of beam a 4 in above the bottom of the beam

$$HSS = [(4,500 \text{ lbs})(6.153 \text{ in}^2)(4.37 \text{ in})]/[(249 \text{ in}^4)(.35 \text{ in})] = 1,388 \text{ psi}$$